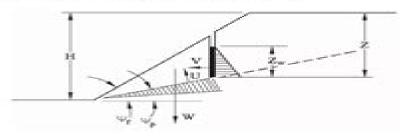
The reinforcement load is applied in the stability analysis either as a single stabilizing element or a series of reinforcing elements to achieve the desired factor of safety. The length of the bolt or cable is dependent on the bond strength (adhesion to the rock) and the discontinuity spacing that forms the deepest part of the block. Tendon lengths can range from 2 to 30 m (6 to 100 ft); however, in the transportation industry, the tendon length rarely exceeds 10 m (30 ft). The detailed requirements for site investigation and analysis of rock cuts are provided in FHWA HI-99-007 Rock Slopes Reference Manual (Munfakh, Wyllie, and Mah 1998)



Witherer:

V = Water force in tension crack.

U = Uplift water force on base of sliding block

W = Weight of sliding block

 ψ_f = Angle of slope face from the horizontal

 ψ_p = Angle of the sliding plane from the horizontal

Z = Height measured from bottom of the tension crack to the crest of the slope

Z_w = Height of water column in tension crack

H = Overall height of the slope

Figure 43. Illustration. Example slope analysis diagram (modified from Hock and Bray 1981).

Rock anchors are usually installed in a grid pattern, where each anchor is the same length and set at a predetermined distance from the surrounding bolts. Following a set pattern can improve the structural stability of an entire rock face, especially for weathered or highly fractured rock. On competent rock masses with large block sizes, engineers typically identify "key blocks" (i.e., blocks of rock that control support for surrounding blocks), then design a bolting pattern around them that makes it more difficult for the surrounding blocks to move. Designing a key block pattern requires engineers to map the three-dimensional fracture orientations, but can decrease the number of rock bolts required to stabilize a slope.

In both tensioned and untensioned anchors, the bearing plate and hex nut are used to distribute the load of the anchor to the rock face; a beveled washer is used to apply the load evenly when the bolt is angled in relation to the rock face. In rock masses with few discontinuities, the plate can be removed and the tendon (bar or cable) cut to allow for the installation of a grout cover or plug. This method is highly contingent on the quality of rock and stability of the rock mass.

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FHWA Research, Development and Technology Implementation Catalog United States. Federal Highway Administration. Implementation Division,1984 **SRIM Index** ,1980 **Government Reports Annual Index** ,1992

Represents current policies and practices of the Illinois Dept of Transportation in the geotechnical aspects of highway engineering Special Report National Research Council (U.S.). Transportation Research Board,1974 Landslides A. Keith Turner,Robert L. Schuster,1996 A guide to using the Public Record Office PRO in England for English or Welsh genealogical research providing an introduction to PRO record classes of interest to North American researchers and identifying PRO records available in North American institutions Includes advice for finding sources of emigration and immigration records with appendices on local record offices in England and Wales and useful addresses Annotation copyright by Book News Inc Portland OR Shored Mechanically Stabilized Earth (SMSE) Wall Systems Design Guidelines Kimberly Finke Morrison,2006 As a Federal Highway Administration design reference for highway projects this report was prepared to enable the engineer to identify and evaluate potential applications of shored mechanically stabilized earth SMSE walls Included in this design guideline are a literature review on similar construction and the results and interpretation of field scale testing centrifuge modeling and numerical modeling of an SMSE wall system Results of the centrifuge modeling and

field scale testing show that reduction of the reinforcement length to as little as 25 percent of the wall height 0 25H provides sufficient wall stability even under a considerably high degree of surcharge loading Using the results of the modeling and field testing research this design guideline recommends a minimum reinforcement length equivalent to as little as 30 percent of the wall height 0 3H for the MSE wall component provided that the MSE reinforcement length is greater than 1 5 m The benefit of attaching reinforcement to the shoring wall is found to be small and is generally not recommended except by way of the upper two layers of reinforcement If possible these layers of reinforcement should overlap the shoring wall and have a total length of 0 6H If this is not possible then these layers should be attached to the shoring wall Internal design requirements of the MSE wall component for an SMSE wall system differ from that of a traditional MSE wall Equations presented in this design guideline have been developed specifically to address these requirements The benefits of increased retaining abilities provided by the shoring wall such as reduction in lateral load acting on the MSE wall component and contribution to global stability are considered in the design process

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Rockfall Hazard Rating Matrix for the State of Ohio Abdul Shakoor, Martin J. Woodard,2005

Colorado Rockfall

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Dynamics, Kinematics, and Geomorphic Evolution of the San Diego, California Coastline Benjamin Todd Benumof,1999 Rockfall Modeling and Attenuator Testing, 1989

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