



Magued Iskander

Behavior of Pipe Piles in Sand

Plugging
and Pore Water Pressure Generating
During Installation
and Loading



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Behavior Of Pipe Piles In Sand Plugging Pore Water Pressure Generation During Installation And Loading Springer Series In Geomechanics And Geoengineering

**Magued Gamal Iskander, Roy E.
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Behavior Of Pipe Piles In Sand Plugging Pore Water Pressure Generation During Installation And Loading

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Behavior of Pipe Piles in Sand Magued Iskander, 2011-01-17 One of the major difficulties in predicting the capacity of pipe piles in sand has resulted from a lack of understanding of the physical processes that control the behavior of piles during installation and loading This monograph presents a detailed blue print for developing experimental facilities necessary to identify these processes These facilities include a unique instrumented double walled pipe pile that is used to delineate the frictional stresses acting against the external and internal surfaces of the pile The pile is fitted with miniature pore pressure transducers to monitor the generation of pore water pressure during installation and loading A fast automatic laboratory pile hammer capable of representing the phenomena that occur during pile driving was also developed and used

Behavior of Pipe Piles in Sand Magued Iskander, 2010 Behavior of Pipe Piles in Sand Magued Iskander, 2011-03-28 One of the major difficulties in predicting the capacity of pipe piles in sand has resulted from a lack of understanding of the physical processes that control the behavior of piles during installation and loading This monograph presents a detailed blue print for developing experimental facilities necessary to identify these processes These facilities include a unique instrumented double walled pipe pile that is used to delineate the frictional stresses acting against the external and internal surfaces of the pile The pile is fitted with miniature pore pressure transducers to monitor the generation of pore water pressure during installation and loading A fast automatic laboratory pile hammer capable of representing the phenomena that occur during pile driving was also developed and used *Stress State Developments During the Installation of Pipe Piles in Sand and Their Influence on the Lateral Load-bearing Behaviour* Philipp Stein, 2023 *Effect of Pile Installation Method on Pipe Pile Behavior in Sands* K. Paik, R. Salgado, 2004 Open ended pipe piles are often used for the foundations of both land and offshore structures because of their relatively low driving resistance In this study calibration chamber tests were conducted on model pipe piles installed in sands with different soil conditions in order to investigate the effects of the pile installation method on penetration parameters and bearing capacity Results of the test program showed that both the hammer blow count necessary to install the piles and the incremental filling ratio IFR which is used to indicate the degree of soil plugging in open ended piles decreased 1 with increasing hammer weight for the same driving energy and 2 with increasing hammer weight at the same fall height The base and shaft load capacities of the piles were observed to increase 1 with increasing hammer weight for the same driving energy and 2 with increasing hammer weight for the same fall height It was also observed that the noise level observed during pile driving decreases 1 as the driving energy decreases and 2 as the hammer weight increases for the same driving energy Model jacked piles were also installed and tested The jacked piles were found to have higher bearing capacities than identical driven piles under similar conditions mostly due to the more effective development of soil plugging in jacking than in driving Pore-water Pressures in Soft to Firm Clay During Driving

of Piles Into Underlying Dense Sand ,1996 During driving of steel piles through soft sensitive clay into very dense sand and gravel pore water pressure responses were monitored As a result of the large length of the piles and also because of the high sensitivity of the soft clays the piles were driven in two stages During the initial stage of driving in the soft clay only very small pore water pressure increases were recorded together with very low pile driving resistances however during the second stage of driving high pore water pressure increases were observed in the clay as soon as the piles penetrated into the underlying very dense sand and gravel It was concluded that the clay deposit was loaded from below as the piles were driven into very dense sand The total stress changes and the resulting pore water pressure changes in the clay were analyzed assuming that the pile driving load was equivalent to a flexible load acting on the surface of an elastic half space which represents the soft clay deposit This interpretation of the pore water pressure increases is important for the assessment of the bearing capacity of engineering structures affected by piles driven through soft soils into very dense deposits The potential for high pore water pressure increases in the clay during undrained loading as well as for volume increases in the dense sand due to pile driving can be predicted from piezocone test data

Plugging of Closed and Open-ended Pipe Piles in Sands Wissam Hadi Shijhait,Mohammed Yousif Fattah,2014-02-28 Plugging of pipe piles is an important phenomenon which is not adequately accounted for in the current design recommendations of open ended pipe piles An open ended pipe pile is said to be plugged when the soil inside the pile moves down with the pile resulting in the pile becoming effectively closed ended Plugging is believed to result in an increase in horizontal stresses between the pile and the surrounding soil which results in an increase in skin friction The present study investigates the effect of plug on pile load capacity and effect of plug removal Different parameters are considered such as pile diameter to length ratio type of installation in sand of different densities removal of plug in three stages 50% 75% and 100% with respect to length of plug Kerbala sand which is used as a foundation soil is poorly graded clean sand The sand was prepared at different densities using a raining technique To simulate the pile load test in the field a new apparatus was manufactured A driving pressing system for pile installation was manufactured in this study The soil plug is removed by a device manufactured to remove the soil column entrapped inside the pipe piles

Pore Water Pressure Increase in Loose Saturated Sand at Level Sites During Three-directional Earthquake Loading Ernesto E. Vicente,1983 *Effects of Membrane Compliance on Pore Water Pressure Generation in Gravelly Sands Under Cyclic Loading* S. Mohsen Haeri,Mohammad Reza Shakeri,2010 The paper deals with an experimental study of the undrained cyclic behavior of a natural coarse sand and gravel deposit located in Tehran a megacity situated on the continental side of the Alborz Mountain in Iran Membrane compliance that plays a significant role in inhibiting redistribution of pore pressure and liquefaction in undrained cyclic triaxial tests performed on coarse granular soils is studied in this paper Currently there is no or little satisfactory method for accounting for this phenomenon for gravelly soils and thus the non compliant cyclic loading resistance of granular soils and the evaluation of the behavior of such material in

natural and in situ state are not easily determined A procedure has been proposed in this paper for consideration of membrane compliance effects on the pore pressure measurements during cyclic loading A method is also introduced to verify the proposed procedure by employing lightly cemented specimens In addition some new correlations for excess pore water pressure ratio against the number of cycles to failure have been presented for a non compliant system under various effective confining and deviatoric cyclic stresses applied on reconstituted and isotropically consolidated specimens Initial liquefaction has been observed during the tests with or without consideration of membrane compliance however the number of cycles to initial liquefaction is higher for the compliant case

Use of case histories in prediction of load-settlement behavior of steel pipe piles in sand Khalifa Salem Alyahyai,1987 *An experimental facility to model the behavior of steel pipe piles in sand* Magued Gamal Iskander,Roy E. Olson,Offshore Technology Research Center,Center for Offshore Technology,1995

Behavior of Piles During Earthquake-induced Lateral Spreading Priyanshu Singh,2002 **Behavior of Dry Sands in Steel Pipe Piles** Hideaki Kishida,1985 **Pore Water Pressure Development in Non-uniform Cyclic Triaxial Tests** Jaw-Nan Wang,1985 *Case History Evaluation of Axial Behavior of Augered-cast-in-place Piles and Pressure-injected Footings* Jie-Ru Chen,1998 The Behavior of Pore Water Pressure in Cohesive Subgrade Soils Jangguen Lee,2003

Capacity and behavior of steel pipe piles in dry sand Omar Mohamed A. Al-Ansari,1999 **Prediction of Pore Water Pressure Buildup and Liquefaction of Sands During Earthquakes by the Cyclic Strain Method** Ricardo Dobry,1982 A cyclic strain approach for evaluating the buildup of excess pore water pressures and the potential for liquefaction of level sandy sites during earthquakes is proposed in this report This strain approach is based on the premise that for undrained loading of sand there is a predictable correlation between cyclic shear strain and excess pore water pressure also that there is a threshold shear strain below which there is no sliding at the contacts between sand particles and no pore water pressure buildup can occur As the result a sand deposit will not develop excess pore pressures if the induced seismic shear strain is less than the threshold strain Both theoretical evidence and experimental verification supporting the cyclic strain approach and the existence of the threshold are presented in the report Based on all there findings a specific design method is proposed for predicting if excess pore pressures will develop at a specific site during a design earthquake

Effect of Prefabricated Vertical Drains on Pore Water Pressure Generation and Dissipation in Liquefiable Sand Antonio Marinucci,2010 Soil improvement methods are used to minimize the consequences of liquefaction by changing the characteristics and or response of a liquefiable soil deposit When considering sites with previous development the options for soil improvement are limited Traditional methods such as compaction and vibratory techniques are difficult to employ because of adverse effects on adjacent structures One potential method for soil improvement against soil liquefaction in developed sites is accelerated drainage through in situ vertical drains Vertical drains expedite the dissipation of excess pore water pressures by reducing the length of the pore water drainage path For more than thirty years vertical gravel

drains or stone columns have been employed to ensure the excess pore water pressure ratio remains below a prescribed maximum value. In recent years the use of prefabricated vertical drains (PVDs) has increased because the drains can be installed with less site disruption than with traditional soil improvement methods. To date little to no field or experimental verification is available regarding the seismic performance of sites treated with PVDs. The effectiveness of PVDs for liquefaction remediation was evaluated via small scale centrifuge testing and full scale field testing. A small scale centrifuge test was performed on an untreated soil deposit and on a soil deposit treated with small scale vertical drains. Compared to the untreated condition the presence of the small scale vertical drains provided numerous benefits including smaller magnitudes of excess pore water pressure generation and buildup, smaller induced cyclic shear strains, reduced times for pore pressure dissipation and smaller permanent horizontal and vertical displacements. In addition full scale in situ field experiments were performed in an untreated soil deposit and in a soil deposit treated with full scale PVDs using a vibrating mandrel as the dynamic source. In the untreated test area the maximum induced excess pore pressure ratio reached about 0.95. In the treated test area the vibratory installation of the first few drains generated significant excess pore pressures, however significant excess pore pressures were not generated during the vibratory installation of additional drains because of the presence of the adjacent drains. Additionally the vibratory installation of the drains caused significant settlement and significantly altered the shear wave velocity of the sand. Dynamic shaking after installation of all of the drains induced small accelerations, small cyclic shear strains and negligible excess pore water pressures in the soil. The results of the field experiment indicate that the prefabricated vertical drains were effective at dissipating excess pore water pressures during shaking and densifying the site.

Two-dimensional Dynamic Pore Pressure Response in Saturated Granular Materials George Michel Marj, 1988. The evaluation of pore pressure response in soils during earthquakes or other types of eye loading conditions has been given considerable attention in recent years. This report concentrates on presenting a comprehensive analysis of the liquefaction problem in granular materials through three major parts: 1. A detailed survey of the analytical methods now available for predicting the pore pressure generation and dissipation in saturated horizontal soil systems subjected to dynamic excitation is conducted. Unfortunately no explicit field pore pressure data are presently available to provide a check on the results obtained by any analysis of this type. However the pore pressure response predicted by such analytical models is in general in accordance with field and experimental observations reported in the technical literature. 2. Since most existing analytical approaches are one dimensional in nature, an analytical model to evaluate two dimensional dynamic pore pressure response in layered granular systems is developed. The model can be equally applied to one dimensional situations. The procedure analytically uncouples the analysis of pore water pressure from the dynamic soil response. Effects of gradient redistribution and external drainage are included. The settlement induced by pore pressure dissipation in the post loading phases is also predicted. Application of the proposed analytical procedure requires only a

knowledge of the characteristics of the dynamic load and the undrained pore pressure generation characteristics of the constitutive sands such data are readily obtained from standard laboratory test procedures 3 The new model is applied to predict dynamic pore pressure changes in two common engineering problems namely the pumping potential of a rigid pavement supported by a granular base course and the settlement and liquefaction potential of a sand deposit supporting a loaded structural foundation when subjected to earthquake shaking In both cases the use of a two dimensional pore pressure analysis is required either because of the drainage conditions or because of the soil initial stress states respectively A parametric evaluation is conducted to illustrate the effect of different parameters on the pore pressure response and associated deformations These include load characteristics drainage conditions and soil properties The results of the study are hoped to serve as an element of judgment in predicting the liquefaction potential and associated deformations of saturated granular materials subjected to dynamic loading

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